# Hydrologic Model Manager

Short Name	DAFLOW	
Long Name	Diffusion Analogy Flow Model	
Description	,	
•	The DATI OW model is a continuous model of flowing a network of and	
Model Type	The DAFLOW model is a continuous model of flow in a network of one- dimensional open channels.	
Model Objectives	The model provides a time series of discharge, flow area, top width, and tributary inflow at node points along a system of one-dimensional open channels. The time series of hydraulic variables can be used as input hydraulics by a transport model, such as the BLTM water quality model. The model is designed to operate with a minimum of field data. It requires no cross sectional information, the channel properties being defined by hydraulic geometry coefficients which have been found to be fairly predictable for a wide range of river sizes throughout the world. The flow must be unidirectional and backwater must not be significant, so a downstream boundary condition is not required. Model accuracy improves with river slope and model time step size. The accuracy is excellent for upland streams and it can be used can be used with good accuracy for time steps as short as one hour as long as the slope is greater than 0.0003.	
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Model Structure	The model solves the diffusion-wave form of the momentum equation. It is, therefore, extremely stable but accuracy degrades as the channel slope and time step size approach zero. The model has been coupled to the USGS MODFLOW groundwater model.  The model has been developed for upland streams where flow in unidirectional and backwater is not significant. As a rule of thumb, the following table gives the approximate minimum slope that should be simulated by DAFLOW for various time steps.  Time step Minimum Slope 5 minute 10 ft/mile; 0.002 1 hour 1.5 ft/mile; 0.0003 6 hour 0.25 ft/mile; 0.00005	
	12 hour 0.1 ft/mile; 0.00002  The model has been used for streams of all sizes ranging from the lower Mississippi to small laboratory flows a few centimeters deep.	
Interception		
Groundwater		
Snowmelt		
Precipitation		
Evapo-transpiration		

Infiltration

### **Model Paramters**

The channel properties, which are defined for each subreach between node points, include the channel slope and the hydraulic geometry coefficients and exponents for area and width. The hydraulic geometry exponents have been found to remain relatively constant for a given river system and even among rivers. The hydraulic geometry coefficients can be estimated from channel resistance (Mannings n) or from the speed of flow waves traversing the system.

# **Spatial Scale**

The river system is represented by a series of branches connected by junctions. Any number of branches may be connected to a junction. Flow must be unidirectional and if more than one branch receives flow from a junction, the percentage of junction flow to enter each branch must be specified. Branches are subdivided into subreaches that are separated by nodes.

### Temporal Scale

The model can be run for any time step ranging from monthly to 0.01 hour. Model accuracy improves with increasing time step. Resolution of course, decreases as the time step increases.

### Input Requirements

The model requires a time series of the inflow at the upstream end of each external branch and tributary inflow that may occur at each interior node.

#### **Computer Requirements**

DAFLOW operates under DOS on any 286, or better, machine. Depending on the application, only 640K of memory and 1.5mb of disk space are required.

# **Model Output**

The DAFLOW produces discharge output at user specified node points and time intervals.

# Parameter Estimatn Model Calibrtn

Hydraulic geometry parameters can be estimated externally, from flow resistance or wave speeds. They may also be determined by use of an optimization routine such that the model results most nearly fit observed discharges at individual sites.

# **Model Testing Verification**

DAFLOW has been tested and used on numerous projects both within and outside of the USGS.

### **Model Sensitivity**

The most important parameter for calibration is the hydraulic geometry coefficient for area (A1), which controls the wave speed at a specific discharge, the timing of the computed hydrograph. Generally the second most important parameter for calibration is the hydraulic geometry exponent for area (A2), that controls the change of wave speed with discharge. The rate of attenuation of the flow peaks is primarily influenced by the hydraulic geometry coefficient for width (W1). The hydraulic geometry exponent for width (W2) controls the change in attenuation with discharge and is often of little consequence, especially for steep channels.

### **Model Reliability**

Model stability and repeatability are excellent. Accuracy decreases with decreasing slope and time step size.

### **Model Application**

The following are references to reports that describe projects that have used the DAFLOW model.

Dorava, J.M., and Meyer, D.F., 1994, Hydrologic hazards in the lower Drift River basin associated with the 1989-1990 eruptions of Redoubt Volcano, Alaska, in Miller, T.P. and Chouet, B.A., eds., The 1989-1990 eruptions of Redoubt Volcano, Alaska: Journal of Volcanology and Geothermal Research, Special Issue, v. 62, no. 1-4, p. 387-407.

Graf, Julia Badal, 1995, Measured and predicted velocity and longitudinal dispersion at steady and unsteady flow, Colorado River, Glen Canyon Dam to Lake Mead: Water Resources Bulletin, American Water Resources Association, Vol. 31, No. 2, April, P. 265-281.

Laenen, Antonius, and John C. Risley, 1997, Precipitation-Runoff and Streamflow-Routing Models for the Willamette River Basin: Oregon, U.S. Geological Survey Water-Resources Investigations, Report 95-4284, 197 pages.

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Nishikawa Tracy, K. S. Paybins, J. A. Izbicki, and E. G. Reichard, "Numerical

model of a tracer test on the Santa Clara River, Ventura county, California," Journal of the American Water Resources Association, v. 35, no. 1, p. 133-142. Paybins, Katherine Schipke, Tracy Nishikawa, John A. Izbicki, and Eric G. Reichard, 1997 "Statistical analysis and mathematical modeling of a tracer test on the Santa Clara River, Ventura County, California," Water-Resources Investigations, Report 97-4275, 19 pages.

Risley, John C., Relations of Tualatin River Water Temperatures to Natural and Human-Caused Factors: U.S. Geological Survey Water-Resources Investigations, Report 97-4071, 143 pages.

Wesolowski, Edwin A., 1999, Simulation of effects of discharging treated wastewater to Sand Creek and Lower Caddo Creek near Ardmore, Oklahoma: U.S. Geological Survey Water-Resources Investigations Report 99-4022, 122 p.

Wiley, J.B., 1993, Simulated flow and solute transport, and migration of a hypothetical soluble-contaminant spill for the New River in the New River Gorge National River, West Virginia, U.S. Geological Survey Water-Resources Investigations Report 93-4105, 39 p.

### **Documentation**

Jobson, H.E., 1989, Users manual for an open-channel streamflow model based on the diffusion analogy: U.S. Geological Survey Water-Resources Investigations 89-4133, 73 p.

Other Comments	
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